1	Upper Mississippi River Flow and Sediment Characteristics and Their Effect on a Harbor
2	Siltation Case
3	Roberto Fernández, S.M. ASCE <sup>1</sup> , Marcelo H. García, Dist. M. ASCE <sup>2</sup> , and Gary Parker, M.ASCE <sup>3</sup>
4	<sup>1</sup> Ph.D. Student, Dept. of Civil and Environmental Engineering, Research Assistant, Ven Te Chow
5	Hydrosystems Laboratory, Univ. of Illinois at Urbana-Champaign. 205 N Mathews Ave. Urbana, IL 61801.
6	(Corresponding author). E-mail: fernan25@illinois.edu
7	<sup>2</sup> M.T. Geoffrey Yeh Chair, Dept. of Civil and Environmental Engineering, Director, Ven Te Chow
8	Hydrosystems Laboratory, University of Illinois at Urbana-Champaign. 205 N. Mathews Ave. Urbana, IL
9	61801. E-mail: mhgarcia@illinois.edu
10	<sup>3</sup> Professor Dept. of Civil and Environmental Engineering, Ven Te Chow Hydrosystems Laboratory and
11	W.H. Johnson Professor, Dept. of Geology, University of Illinois at Urbana-Champaign. 205 N.
12	Mathews Ave. Urbana, IL 61801. E-mail: parkerg@illinois.edu
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#### 14 Abstract

Upper Mississippi River flow and sediment characteristics downstream of St. Louis, MO are 15 presented in this study. Available and measured data were used to assess a harbor siltation case 16 17 and dredging needs. Such data are also useful to researchers and engineers conducting work in 18 the Mississippi River, and large rivers in general. Flows were characterized in terms of the mean 19 annual hydrograph, the flow duration curve and the mean annual, dominant and effective 20 discharges. Suspended and bed material sediments were characterized by grain size distributions (GSDs). Suspended sediment concentrations were characterized with a sediment-rating curve, a 21 22 mean annual sediment-graph and a duration curve. The results of the analyses were used to assess harbor sedimentation by comparing GSDs of harbor bed samples with those observed in 23 24 the river. Bathymetric surveys were used to determine rates and occurrence of sedimentation. 25 The analyses showed that harbor siltation correlates with river conditions, and is driven by wash 26 load in the river, which enters the harbor in suspension and deposits along the bottom due to the lack of flow-through velocities high enough to keep the fine sediments in suspension. 27

#### 28 Key words

Upper Mississippi River, Wash Load, Harbor, Siltation, Dominant discharge, Effective discharge
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31 Introduction

Sediment characteristics and loads in the Mississippi River have been the subject of numerous 32 studies in the past decades. Their relation with land loss (e.g. Kesel 1989, 1988; van Heerden 33 and DeRouen 1997) has been one of the key factors driving the need to better assess the 34 35 sediment loads in the river and its tributaries. Some recent studies on sediment load trends in the river basin (e.g. Horowitz 2010; Meade and Moody 2010; Blevins 2006) suggest that sediment 36 37 loads are declining. In spite of this, potential for building river diversions that would carry sediment 38 to certain locations along the shoreline to prevent further land loss at the Mississippi River delta, and along coastal Louisiana, has been recognized (e.g. Paola et al. 2011; Allison and Meselhe 39 40 2010). The amount of sediment diverted is a function of the flow and sediment load in the river (Dutta et al. 2017) and proper quantification of both variables is required. As a result, research 41 42 has mainly focused on the Missouri and Ohio Rivers, which are responsible for the largest tributary sediment loads (Heimann et al. 2011), or on the lower Mississippi River sediment loads (e.g. 43 Thorne et al. 2015). This study presents a characterization of the flow and sediment in the Upper 44 Mississippi River at St. Louis, MO, to contribute to such efforts and facilitate future water and 45 46 sediment diversions. In addition, the analysis and results are used to assess a siltation problem 47 at a harbor built in 2006 on the right bank of the Upper Mississippi River close to Ste. Genevieve, MO. 48

Siltation is the process by which fine sediment particles suspended in a water body settle and deposit on the bed. Harbor siltation is a common problem throughout the world, and different sediment management strategies have been proposed in the literature to prevent it or slow it down and therefore reduce the necessity for dredging (e.g. Kirby 2011; Winterwerp 2005; van Schijndel and Kranenburg 1998; Berlamont 1989). Successful implementations of such strategies have
been well documented (e.g. Kuijper et al. 2005; Winterwerp et al. 1994) but in spite of these, not
all harbors, and especially not all riverine harbors, are designed with the potential consequences
of siltation in mind.

Two distinct foci were established to assess the siltation problem which began a few months after
the harbor started operating in 2007, namely, (i) the harbor itself and (ii) the Upper Mississippi
River between St. Louis, MO and Chester, IL. Specific tasks involved the following:

- Harbor sediment sample collection and analysis. Samples were taken to the Ven Te Chow
   Hydrosystems Laboratory at the University of Illinois at Urbana-Champaign for their
   analysis.
- Determination of siltation volumes and average siltation rates from harbor bathymetric
   surveys provided by the harbor's owners.
- Characterization of the Upper Mississippi River flow conditions using data from
   neighboring United States Geological Survey (USGS) gaging stations. The data are
   available through the National Water Information System (USGS 2017).
- Characterization of the material in suspension and deposited on the bed in the Upper
   Mississippi River. These data are also available through the National Water Information
   System (USGS 2017).

These tasks set the structure of this paper. It is divided into five sections, with this introductory section being the first. The second section describes the harbor, its geometry and location, and presents measured siltation rates and patterns and bed sediment characteristics. The third section focuses on the characteristics of the flow and sediment in the reach of the Upper Mississippi River in the vicinity of the harbor. The fourth section presents the key findings from the tasks enumerated above, and provides answers regarding the following questions. (i) What is the source of the sediment responsible for siltation inside the harbor? (ii) When are these sediments most likely to be deposited? (iii) How does siltation relate to the hydraulic conditions in the river? The fifth and last section summarizes the conclusions of the analysis.

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# 81 Site Characteristics

### 82 Harbor Location and Dimensions

The harbor is located between Upper Mississippi River Miles 138 and 139 as shown in Fig. 1. The site is approximately 65 km downstream of St. Louis, MO and 45 km upstream of Chester, IL. With a design maximum depth of 17 m, the harbor is 500 m long and 200 m wide. The actual depth and volume of water in the harbor varies with river stage.

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### 88 Measured Harbor Siltation Rates and Patterns

Siltation volumes inside the harbor are available for different dates due to bathymetric 89 measurements conducted by the dredging company that was hired by the harbor's owners. Table 90 1 shows the siltation volumes measured between February of 2008 and June of 2010. The 91 number of days between surveys and the harbor's maximum surface area (10,000 m<sup>2</sup>) were used 92 93 to compute mean siltation rates, equivalent deposit thicknesses and daily siltation depths. Siltation 94 periods (dredging campaigns) are reflected by an increase (a decrease) in the excess volume of 95 sediment reported in Table 1. This volume corresponds to the difference between a given bathymetric survey and the harbor design geometry. 96

97 The patterns of sediment deposition inside the harbor are shown in Fig. 2. The original harbor 98 bathymetry is shown alongside bathymetric surveys conducted in January 26<sup>th</sup>, March 10<sup>th</sup>, June 99 04<sup>th</sup> and July 28<sup>th</sup> of 2009. A small insert is included to indicate the relative position of the harbor 100 with respect to the Mississippi River and the direction of flow. An aerial image of the harbor, taken 101 from Google Earth, is also included. The amount of sediment deposited on the bed of the harbor in January 26<sup>th</sup>, 2009 corresponds to material that had accumulated previously and was not
 removed during the dredging efforts conducted in the second semester of 2008 (see Table 1).

#### 104 Harbor Bed Sediment Characteristics

#### 105 Grain Size Distributions

Samples from the harbor bed were extracted during two separate campaigns conducted in 106 December of 2010 and December of 2011. Fig. 3 shows the sampling locations from both 107 campaigns, and Table 2 indicates the size of the samples. Grain size distribution analyses for 108 samples 1 and 5 were conducted according to the "Standard Test Method for Particle-Size 109 Analysis of Soils" (ASTM 2002), referred to herein as the hydrometer method, and with the LISST-110 111 ST settling tube (Pedocchi and Garcia 2006) for comparison. Results from both methods compared well (Fernández et al. 2012) and therefore samples 1-9 were analyzed only with the 112 LISST-ST for simplicity. Samples from the 2011 campaign and from a drum collected in 2010 113 were analyzed with the hydrometer method. To assess the role of flocculation in the harbor, the 114 analyses were conducted on samples dispersed with sodium hexametaphosphate (NaPO<sub>3</sub>)<sub>6</sub>, as 115 116 well as on non-dispersed samples. Results are shown in Fig. 4.

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#### 118 Settling Velocities

119 Results from the grain size distribution analyses on dispersed and non-dispersed samples 120 suggest that flocculation occurs inside the harbor. Flocculation may lead to enhanced siltation rates through faster settling of material. The settling velocity of the flocs depends on the size, 121 density and shape, which in turn are governed by inter-particle collision frequency (Mehta and 122 McAnally 2008). Krone (1962) suggested that since particle collision frequency depends on the 123 concentration of particles in suspension, suspended particle concentration may be used as a 124 125 surrogate to estimate the floc settling velocity. Wolanski et al. (1989) proposed dividing the settling process into four zones; empirical relations have been developed to express settling velocity in 126 127 each zone as a function of suspension concentration. Table 3 shows the different zones; an empirical relation proposed by Hwang (1989) to estimate settling velocities in each zone is givenby Eq. 1.

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$$w_{s} = \begin{cases} w_{sf} & C < C_{1} \\ a_{w} \frac{C^{n_{w}}}{(C^{2} + b_{w}^{2})^{m_{w}}} & C_{1} < C < C_{2} \\ C_{2} < C < C_{3} \\ \sim negligible & C_{3} < C \end{cases}$$
 Eq. 1

 $w_{sf}$  = free settling velocity;

C = volume suspension concentration;

 $a_w$  = velocity scaling coefficient;

 $n_w$  = flocculation settling exponent;

 $b_w$  = hindered settling coefficient;

- $m_w$  = hindered settling exponent;
- $C_1 C_3$  = zone concentration limits as defined in Table 3;

When the suspension concentration is below a value  $C_1$ , free settling occurs and the settling velocity corresponds with the one each particle would have in the absence of other particles. Particles are far away from each other and no flocculation occurs. As concentrations increase, flocculation begins to occur and therefore the original grains begin to form flocs which have higher settling velocities. This process of flocculation settling continues up to a concentration C2, which corresponds with the maximum settling velocity (w<sub>sm</sub>). Above C<sub>2</sub>, the concentration becomes so high that the flocs have trouble settling and begin to collide with each other. Settling becomes hindered and could be thought of as a condition where water is trying to escape the pore space as sediment settles down. If concentration continues to increase and reaches a value C<sub>3</sub>, the process turns into a consolidation process rather than a settling one. Zone concentration limits and coefficients are not universal and depend on the sediment type and grain size distribution, as 

well as the environmental conditions in which the settling process takes place, such as salinityand turbulence or the lack thereof.

153 The settling velocity of the material found in the harbor was determined by conducting the settling column experiments first described by McLaughlin (1959) and later improved by Ross (1988). Fig. 154 5 shows the settling column used. It is 0.10 m in diameter and 1.9 m high, and has 5 mm sampling 155 tubes located at the following elevations above the bed: 0.06 m, 0.16 m, 0.31 m, 0.51 m, 0.72 m, 156 0.93 m, 1.13 m, 1.33 m and 1.54 m. The design of the column is based on the one developed by 157 Lott (1987), and the experimental procedure followed the one described by Ross (1988). The 158 following five different initial concentration conditions:  $C_0 = 1 \text{ g/L}$ , 5 g/L, 10 g/L, 15 g/L, and 25 g/L 159 160 were used. Results from the experiments are shown in Fig. 6 along with a curve fit with Eq.1; the resulting coefficients are shown in Table 4. 161

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#### 163 Upper Mississippi River Characteristics in the Near-Harbor Area

#### 164 Available Data

Data available at USGS gaging stations 07010000 at St. Louis, MO and 07020500 at Chester, IL were used to characterize the Mississippi River in the vicinity of the harbor. A summary of the data is shown in Table 5. Given that no significant tributaries flow into the Mississippi River between St. Louis, MO and Chester, IL a preliminary analysis showed that for the matching period of record July 1942 – November 2011 the flow conditions, on average, differ by less than 1% (Fernández et al. 2012). Therefore, all analyses related to river data presented hereafter only used the information recorded at St. Louis, MO.

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#### 173 Flow Discharge and Suspended Sediment Concentrations

Flow discharge and sediment concentrations in the Mississippi River at St. Louis, MO for the period October 1<sup>st</sup>, 1980 to September 30<sup>th</sup>, 2011 are shown in Fig. 7. Historic mean flows at St. Louis, MO computed for different periods are shown in Table 6. Values reported therein indicate that the mean flow in the Mississippi River for the period beginning when the harbor started operation (2007) and ending in 2011 has been approximately 56% larger than what it had been over the period beginning in 1861 and ending in 2011 and 33% larger than what it had been over the period beginning in 1980 and ending in 2011.

A suspended sediment concentration rating curve determined from the 11,285 measurements available for the 30 year period is shown in Fig. 8. A power law curve shown in Eq. 2 and on the lower right of Fig. 8, was fit to the data (solid line) and envelopes indicating concentration values equal to 0.2, 0.5, 2.0 and 5.0 times the values estimated with the power curve fit to the data are indicated with dashed lines. Although the data shows scatter, 80.4% (99.3%) of the data lie inside the envelopes for 0.5-2.0 (0.2-5.0) times the value obtained with the power law relation.

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$$C = 1.022e^{-05}Q^{1.1641} \qquad \qquad Eq.2$$

188 where

189 C = suspended sediment concentration [g/L]; and

190 Q =flow discharge [m<sup>3</sup>/s].

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#### 192 Suspended and Bed Material Sediment Characteristics

Grain size distributions for the sediments in the Mississippi River at St. Louis, MO were available 193 194 as part of U S Geological Survey field/lab water quality samples (Table 5). Fig. 9 shows a total of 108 grain size distributions of the material traveling as suspended load and Fig. 10 shows a total 195 of 114 grain size distributions for the material found on the bed of the Mississippi River at St. 196 197 Louis, MO. The solid black line represents the median grain size distribution curve, and the dashed lines correspond to the 75<sup>th</sup> and 25<sup>th</sup> percentiles. The sediment size for which 50% of the 198 grains are smaller is 0.008 mm for the material traveling as suspended load and 0.44mm for the 199 200 material found on the bed of the river.

Mean Annual Hydrograph, and Suspended Sediment Concentrations and Duration Curves The mean annual flow hydrograph and mean annual sediment concentrations are shown in Fig. 11. A black dashed line spike can be seen in the sediment concentration hydrograph during late February. That line corresponds to the 30-year daily average concentrations but it is significantly biased by an extreme event that occurred in February of 1985, as shown in Table 7. If the values for those days are not included in the averaging process, the curve takes the shape of the solid line, which was taken as the representative mean annual sediment concentration curve herein.

Fig. 12 shows the flow duration curve and the suspended sediment concentration duration curve based on the mean annual data in Fig. 11. Flows (concentrations) are lower than 5,000 m<sup>3</sup>/s (0.27 g/L) for half of the year and higher than 8,000 m<sup>3</sup>/s (0.43 g/L) for 30% of the year. The remaining 20% of the time covers the periods in which flows and suspended sediment concentrations increase (decrease) rapidly between mid-February and mid-March (mid-July and mid-August).

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### 215 Characteristic Flow Discharges

Different definitions of a constant characteristic discharge that would be capable of producing the 216 same channel morphologies observed in a river under varying flow conditions have been 217 218 proposed in the literature. Some of these definitions are related to channel equilibrium conditions 219 (e.g. Inglis 1947), meander wavelengths (e.g. Ackers and Charlton 1970), stream bankfull 220 geometry (e.g. Wilkerson and Parker 2011; Nixon 1959), exceedance probability (e.g. Blench 1956; Leopold and Maddock 1953) or sediment transport capabilities (e.g. Terrell and Borland 221 222 1958). These concepts are typically applied in relation to geomorphic processes and their effect on channel geometry. 223

In this study, the concept of characteristic discharges is adapted to assess the flows responsible for the sediment loads in the Upper Mississippi River. Specifically, the concepts of dominant and effective discharge are used due to their relation with sediment loads in the river without consideration for morphological implications. The dominant discharge is defined here as the flow 228 that, if sustained throughout a period of time, would produce the same mean sediment discharge observed during that period under varying flow conditions. The effective discharge is defined here 229 230 as the one carrying the largest volume of sediment in the river. This definition is based on the bed-generative discharge concept first proposed by Schaffernak (1916, 1922), and its 231 computation follows the approach described by Biedenharn et al. (2000). The method has been 232 used and described by different authors (e.g. Garde and Ranga Raju 1977; Gandolfo 1940) but 233 other authors refer to it as the dominant discharge (e.g. Thomas and Benson 1966). It is not the 234 objective of this study to provide clarification and comparison between available definitions; the 235 236 reader is referred to Soar and Thorne (2011) for a recent review on the subject.

Using the data available for the 1981-2011 period, the mean annual suspended sediment concentration was determined and the dominant discharge was back calculated with the sediment-rating curve shown in Fig. 8 and Eq. 2. The values obtained are 0.337g/L for the mean concentration and 7,608 m<sup>3</sup>/s for the dominant discharge.

The effective discharge computation is shown in Fig. 13. The resulting value is 9,582 m<sup>3</sup>/s, which 241 corresponds to the maximum value of the curve of weighted contributions (right panel) obtained 242 from the product of the flow frequency curve (left panel) and the sediment rating curve (middle 243 244 panel). Other local maxima may be seen in the curve. These represent the discharges responsible 245 for carrying large sediment volumes. As is often the case, the result obtained has two distinctive peaks, indicating that a frequent discharge carrying a relatively small sediment load for a long 246 time is almost as effective as an infrequent discharge carrying a large amount of sediment over a 247 shorter period of time. Using the rating curve in Fig. 8, the suspended sediment concentration 248 associated with the effective discharge was obtained. The resulting value was 0.441 g/L. 249

#### 251 Key Findings and Discussion

### 252 What is the source of the sediment responsible for siltation inside the harbor?

#### 253 Origin based on grain size distributions and sedimentation patterns

254 The sediment size analyses from the river and the harbor are summarized in Fig. 14; median  $D_{50}$ values are shown in Table 8. Harbor bed sediments are slightly coarser than the material that is 255 256 carried in suspension by the Upper Mississippi River at St. Louis, MO but are significantly finer than the material in the bed of the river, suggesting that the sediment source is likely to be the 257 suspended sediment in the river. The sedimentation patterns inside the harbor also shed light on 258 259 the origin of the sediment. As shown in Fig. 2, siltation blankets the entire bed of the harbor. The relatively uniform thickness of the deposited sediment observed in the March and June 260 bathymetries is due to a combination of two factors: the fine-grained nature of the deposited 261 sediment, and barge traffic (approximately 20 barges per day), which can under some conditions 262 cause resuspension and redistribution due to propeller wash (Garcia et al. 1999). Although 263 264 coarser materials were found close to the entrance, all sediments were significantly finer than the Upper Mississippi River bed material. 265

#### 266 Suspended sediment dynamics within the harbor

The sediment that enters the harbor in suspension is deposited first on the perimeter of the harbor where the flow velocities and shear stresses, even in the presence of barge traffic, approach zero. Sediment deposits preferentially along these zones and then builds up uniformly from the edges towards the middle of the harbor. The siltation patterns shown in Fig. 2 show some zones that are lower in elevation in the south section close to the entrance. These areas have likely been scoured due to barge traffic going in and out of the harbor.

The settling velocities determined in the experiments (Fig. 5) and shown in Fig. 6 range between 1e-6 to 1e-3 m/s, with the largest values associated with larger suspended sediment concentrations at which flocculation occurs. Although the concentrations in the Mississippi River rarely exceed 2g/L (Fig. 8), it is possible that concentrations may exceed this value inside the harbor as the sediment settles to the bottom. This is most likely to prevail during periods when the harbor is not operating at full capacity. The presence of a bar-like feature on the east side of the harbor on the July 29<sup>th</sup> bathymetry is also thought to be related to barge traffic redistribution of sediments, since most of the barge traffic occurs through the southern part of the harbor and towards the west and north west sections.

#### 282 When are the sediments most likely to be deposited in the harbor?

Harbor siltation volumes and rates are shown in Fig. 15. The black solid line corresponds with the 283 284 volumes of sediment above the design conditions of the harbor. The values are divided by 20 so as to plot this variable using the same axis limits as the flow discharge, and to clearly present the 285 286 salient trends. In the three cases where the volume of sediment in the harbor increases, the period 287 corresponds to late February or early March to late July or early August. (Decreases are caused almost solely by dredging.) This timeframe corresponds to the spring and early summer months; 288 siltation rates within this period can be as high as 1.2 cm/m<sup>2</sup>/day, as indicated by the red dashed 289 290 line.

# 291 Applicability of the dominant and effective discharge concepts

Typically, the dominant and effective discharge concepts are not meant to be used in rivers where 292 293 the majority of the material transported corresponds to silt and clay sizes (i.e. wash load). The 294 main reason for this is that wash load does not correlate with flow discharge and therefore, as 295 long as the sediment is available, the river will transport it regardless of the flow magnitude. Fig. 296 14 shows that more than 80% of the material traveling in suspension in the Upper Mississippi River corresponds to wash load. However, Fig. 8 and Fig. 11 show that wash load in the 297 298 Mississippi River, as defined using e.g. the 62.5 µm cutoff criterion (River Research Council, 2007), does indeed positively correlate with discharge to a surprising degree. The trends shown 299 by both variables in Fig. 11 are remarkably similar, and more than 80% of the suspended sediment 300 301 concentration data shown in Fig. 8 lies within envelopes of 0.5-2.0 times the value estimated with 302 the sediment rating curve. A possible explanation for this behavior is given below.

During late February and early March, snowmelt takes place and river flows increase. At the same time, fine sediment from bare agricultural land is carried by runoff into the river and transported as wash load. This phenomenon is sustained throughout the growing season, and is enhanced by rainfall in the spring and early summer. Once the crops are established and precipitation diminishes (late summer), fine sediment availability is reduced and both the flows and suspended sediment concentrations in the river return to their base flow patterns. The mean annual hydrograph shown in Fig. 11 reflects these processes.

Snowmelt followed by spring and early summer precipitation contribute to the flow magnitude and the availability of sediment due to bare agricultural land in the Upper Mississippi River basin, thus creating conditions in which fine sediment availability matches the period of high flows. High flows do not necessarily cause larger sediment transport, but are correlated due to the characteristics of the river basin. The dominant and effective discharge concepts may be applied in this and other river basins where sediment availability matches the period of high flows even though the relation between the two variables is not strictly causal.

### 317 How does siltation relate to the hydraulic conditions in the river?

Table 9 summarizes the results obtained for the characteristic discharges, the number of days for which they are exceeded and the associated suspended sediment concentrations. Comparison of the characteristic discharges with the mean annual hydrograph and mean annual suspended sediment concentrations shown in Fig. 11 suggest that the Mississippi River carries larger sediment volumes between the end of February and early August than otherwise.

The dominant discharge is exceeded for 120 days between mid-March and mid-July, and the effective discharge is exceeded only for a few days in April and all of May. Siltation volumes and siltation rates are shown in Table 1 and Fig. 15; they are greatest in periods including these months. Although bathymetric survey dates allow assessment of the silting process over the period between February and August, lack of data for the months of April and May impede determining if harbor siltation occurs mostly during early or late spring, summer or both. Nonetheless, the process of siltation is clearly related to flow conditions in the river. The data show that whenever suspended sediment concentrations at St. Louis, MO are above 0.44 g/L, large siltation volumes inside the harbor are possible. According to Fig. 12, these concentrations are met during 30% of the year.

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# **Potential Effect of Barge Traffic and Towboat Operations on Harbor Siltation**

335 Studies on the effect of towboat navigation and barge tows under typical conditions of Upper Mississippi River traffic have shown that bed shear stresses under such conditions deviate from 336 337 those expected under steady-uniform flow. More specifically, higher shear stresses are 338 associated with the passage of the tow and the stern of the barge tow (Rodriguez et al. 2002; Garcia et al. 1999, 1998). Barge traffic in and out of the harbor plays an important role in sediment 339 340 resuspension. The harbor is directly open to the Mississippi River, but has no through-flow discharge and thus acts as a sediment trap. Towboats and barges that enter for loading and 341 unloading operations resuspend the sediment in the harbor, but even with the small settling 342 velocities measured in the laboratory and reported in Fig. 6, such resuspension does not seem to 343 contribute substantially toward keeping sediment from settling inside the harbor. As shown in 344 345 Table 1 and Fig. 15, between the months of July and December of 2009, the excess volume of 346 sediment in the harbor decreased and no dredging efforts took place. This suggests that in those 347 months in which Upper Mississippi River flow discharge and suspended sediment concentrations return to base levels, sediment resuspended by towboats and barges may leave the harbor. This 348 349 observed decrease, however, corresponds to only an insignificant amount of sediment compared to the amount that comes into the harbor during the spring and summer months. 350

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352 **Conclusions** 

Flow and sediments in the Upper Mississippi River were characterized with information available at the USGS gaging station in St. Louis, MO. The most relevant results of our analysis are as follows.

- The correlation between wash load and flow discharge in the Upper Mississippi River is
   due to the characteristics of the basin, namely, snowmelt followed by spring and early
   summer precipitation over bare agricultural land that create conditions in which fine
   sediment availability matches the period of high flows.
- 360
   2. The dominant and effective discharge concepts may be applied to the Upper Mississippi
   361
   River and similar basins where these conditions are met.
- 362 3. The D<sub>50</sub> for the material carried in suspension by the Mississippi River at St. Louis, MO is
  363 0.008 mm and for the material found on the bed it is 0.44 mm.
- 364
   4. Settling velocities for the material carried in suspension by the Mississippi River in St.
   365
   Louis, MO are between 1e-6 to 5e-4 m/s with the largest values associated with larger
   366
   suspended sediment concentrations where flocculation is possible.

367 Comparison of the Upper Mississippi River data with laboratory results of harbor bed samples368 and bathymetric survey data leads to the following findings:

- 369 5. Sediment deposited in the harbor is wash load from the Upper Mississippi River that enters
  370 the harbor in suspension and deposits due to the lack of flow-through inside;
- 371 6. Towboat and barge operations resuspend sediment, but their effect on preventing siltation
  372 is negligible in spite of the small settling velocities;
- 373 7. Flow conditions in the Upper Mississippi River in the period between Mid-March and Mid-
- July correlate with high siltation rates inside the harbor; the analysis suggest (but in the
- 375 absence of specific bathymetric data does not prove) that large siltation rates are possible
- in the month of May when the effective discharge in the Mississippi River is exceeded.

#### 377 Acknowledgements

The authors would like to thank Fernando Valencia, GV Terminal Manager at the time this study was conducted. Participation of all authors in this study was possible thanks to financial support provided by Holcim US through its St. Genevieve Plant in Missouri. The authors would also like to thank the anonymous reviewers and the associate editor for their valuable feedback.

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# 383 References

- Ackers, P., and Charlton, F.G. (1970). Dimensional Analysis of Alluvial Channels with Special
- Reference to Meander Length. Journal of Hydraulic Research, Vol. 8, No. 3. pp. 287-316.
- Allison, M. A., and Meselhe, E.A. (2010). The use of large water and sediment diversions in the
- lower Mississippi River (Louisiana) for coastal restoration. Journal of Hydrology 387(3), pp. 346360.
- 389 American Society for Testing and Materials ASTM (2002). Standard Test Method for Particle-
- 390 Size Analysis of Soils. ASTM International, PA, United States, pp. 1-8
- Biedenharn, D. S., Copeland, R. R., Thorne, C. R., Soar, P., Hey, R.D., and Watson, C.C.
- 392 (2000). Effective Discharge Calculation: A Practical Guide US Army Corps of Engineers,
- 393 ERDC/CHL; TR-00-15, 63 pages.
- 394 Berlamont, J. (1989). Pumping fluid mud: theoretical and experimental considerations. Journal
- of Coastal Research, Special Issue No. 5. High Concentration Cohesive Sediment Transport.
- 396 pp. 195-205, Fort Lauderdale, Fl.
- Blench, T. (1956). Regime Bahaviour of Canals and rivers. Butterworths Scientific Publication,
  London, 87 pages.
- Blevins, D. W. (2006) The response of suspended sediment, turbidity, and velocity to historical
  alterations of the Missouri River: U.S. Geological Survey Circular 1301, 8 p.

- Dutta, S., Wand, C., Tassi, P. and Garcia, M.H. (2017) Three-Dimensional Numerical Modeling
  of the Bulle-Effect: the non-linear distribution of near-bed sediment at fluvial diversions. Earth
  Surface Processes and Landforms. DOI: 10.1002/esp.4186
- 404 Fernández, R., Santacruz, S., Tokyay, T., Waratuke, A., Garcia, M.H., and Parker, G. (2012).
- 405 Holcim Ste. Genevieve Missouri Harbor Siltation Study. Civil Engineering Studies. Hydraulic
- Engineering Series No. 102. UIUC-ENG-2015-2006. ISSN 0442-1744. 71 pages. Champaign,
  IL, United States.
- 408 Gandolfo, J. (1940). Estudio de la evolución fluvial que determina el endicamiento del río San
- 409 Juan. (in Spanish). Universidad Nacional de la Plata, Publicaciones de la Facultad de Ciencias
- 410 Fisicomatemáticas, La Plata, Argentina.
- 411 Garcia, M.H., Admiraal, D.M., and Rodriguez, J. (1999) Laboratory Experiments on Navigation-
- 412 Induced Bed Shear Stresses and Sediment Resuspension. Journal of Sediment Research,

413 14(2), pp. 303-317.

- 414 Garcia, M.H., Admiraal, D.M., Rodriguez, J., and Lopez, F. (1998) Navigation-Induced Bed
- 415 Shear Stresses: Laboratory Measurements, Data Analysis, and Field Application. Civil
- 416 Engineering Studies. Hydraulic Engineering Series No. 056. UIUC-ENG-98-2002. ISSN 0442-
- 417 1744. 139 pages. Champaign, IL, United States.
- Garde, R. J., and Ranga Raju, K.G. (2006) Mechanics of Sediment Transportation and Alluvial
  Stream Problems. New Age International Limited. Third Ed. Reprint 2006. New Delhi, India, pp.
  340-343.
- Heimann, D.C., Sprague, L.A., and Blevins, D.W. (2011) Trends in suspended-sediment loads
  and concentrations in the Mississippi River Basin, 1950–2009: U.S. Geological Survey Scientific
  Investigations Report 2011–5200, 33 p.

- 424 Horowitz, A.J. (2010) A quarter century of declining suspended sediment fluxes in the
- 425 Mississippi River and the effect of the 1993 flood: Hydrological Processes, v. 24, p.13–34.
- 426 Hwang, K.-N. (1989) Erodibility of fine sediment in wave dominated environments. MS Thesis,
- 427 University of Florida, Gainsville, Florida. 159 pages.
- Inglis, C.C. (1947) Meanders and Their Bearing on River Training. Proceedings of the Institution
  of Civil Engineers, Maritime Paper No. 7, London.
- 430 Kesel, R.H. (1988) The decline in the suspended sediment load of the lower Mississippi River
- 431 and its influence on adjacent wetlands: Environmental Geology and Water Sciences, v. 11, p.
- 432 271–281.
- 433 Kesel, R.H. (1989) The role of the Mississippi River in wetland loss in southeastern Louisiana,
- 434 USA: Environmental Geology and Water Sciences, v. 13, p. 183–193.
- 435 Kirby, R. (2011) Minimising harbour siltation findings of PIANC Working Group 43. Ocean
- 436 Dynamics, 61(2-3), pp. 233-244.
- 437 Krone, R.B. (1962) Flume studies of the transport of sediment in estuarial shoaling processes.
- 438 Final Report, hydraulic engineering Laboratory and Sanitary Engineering Research Laboratory,
- 439 University of California, Berkeley, California. 110 pages.
- 440 Kuijper, C., Christiansen, H., Cornelisse, J. M., and Winterwerp, J.C. (2005). Reducing Harbor
- Siltation. II: Case Study of Parkhafen in Hamburg. Journal of Hydraulic Engineering 131(6) pp.
- 442 267-276. DOI: 10.1061/(ASCE)0733-950X(2005)131:6(267)
- Leopold, L.B., and Maddock, T. (1953) The Hydraulic Geometry of Stream channels and Some
- 444 Physiographic Implications. USGS Professional Paper 252, Washington D.C., 64 pages.
- Lott, J. W. (1987). Laboratory Study on the Behavior of Turbidity Current in a Closed-end
- 446 Channel. M.S. Thesis. University of Florida Gainesville, Florida.

- McLaughlin, R.T. Jr. (1959). The Settling Properties of Suspensions. Journal of the Hydraulics
  Division, American Society of Civil Engineers. Vol. 85, No. HY 12, pp. 9-41.
- 449 Meade, R.H., and Moody, J.A. (2010) Causes for the decline of suspended-sediment discharge
- in the Mississippi River system, 1940–2007: Hydrological Processes, v. 24, p. 35–49.
- 451 Mehta, A. J., and McAnally, W.H. (2008) Fine-Grained Sediment Transport in Sedimentation
- 452 Engineering Processes, Measurements, Modeling and Practice (Garcia, M.H. ed.) American
- 453 Society of Civil Engineers. ASCE Manuals and Reports on Engineering Practice No. 110, pp.
- 454 253-306.
- 455 Nixon, M.A. (1959) Study of the Bankful Discharges of Rivers in England and Wales,
- 456 Proceedings of the Institution of Civil Engineers, London, pp. 157-174.
- 457 Paola, C., Twilley, R.R., Edmonds, D.A., Kim, W., Mohrig, D., Parker, G., Viparelli, E. and
- 458 Voller, V. R. (2011) Natural Processes in Delta Restoration: Application to the Mississippi Delta.
- 459 Annual Review of Marine Science, Vol. 3 p. 67-91.
- 460 Pedocchi, F., and and Garcia, M.H. (2006) Evaluation of the LISST-ST instrument for
- 461 suspended particle size distribution and settling velocity measurements. Continental Shelf
- 462 Research, Vol 26, pp. 943-958. doi: 10.1016/j.csr.2006.03.006
- 463 River Research Council, Division on Earth and Life Studies, Water Science and Technology
- 464 Board, Committee on River Science at the U.S. Geological Survey. (2007) River Science at the
- 465 U.S. Geological Survey. National Academic Press, p.87.
- 466 Rodriguez, J., Admiraal, D.M., Lopez, F., and Garcia, M.H. (2002) Unsteady Bed Shear
- 467 Stresses Induced by Navigation: Laboratory Observations. Journal of Hydraulic Engineering,
- 468 10.1061/(ASCE)0733-9429(2002) 128(5), pp. 515-526.
- 469 Ross, M.A. (1988) Vertical Structure of Estuarine Fine Sediment Suspensions. PhD Thesis,
- 470 University of Florida, United States, 112 pages.

- 471 Schaffernak (1916) Die Theorie des Geschiebebetriebes und ihre Anwendung. Zeitschrift des
- 472 oesterrichischen Ingenieur und Architekten Vereines. Wien. Nr. 68.
- 473 Schaffernak (1922) Neue Grundlagen für die Berechnung der Geschiebeführung in Flussläufen.
- 474 Verlag: Franz Deutike. Leipzig-Wien.
- 475 Soar, P.J., and Thorne, C.R. (2011) Design Discharge for River Restoration in Stream
- 476 Restoration in Dynamic Fluvial Systems (eds. Simon, A., Bennett, S.J. and Castro, J.M.),
- 477 American Geophysical Union, Washington D.C. doi: 10.1029/2010GM001009.
- 478 Terrell, P. W., and Borland, W. M., (1958) Design of Stable Canals and Channels in Erodible
- 479 Material. Transactions of the American society of Civil Engineers, 123(1), pp. 101-115.
- 480 Thomas, D. M., Benson, M.A. (1966) A definition of dominant discharge. International
- 481 Association of Scientific Hydrology. Bulletin 11:2, 76-80.
- 482 Thorne, C., Knuuti, K., Harmar, O., Watson, C., Clifford, N., and Biedenharn, D. (2015) Recent
- and Historical Sediment Loads in the Lower Mississippi River. Proceedings of the 3<sup>rd</sup> Joint
- 484 Federal Interagency Conference on Sedimentation and Hydrologic Modeling. Reno, Nevada,
- 485 USA.
- 486 US Army Corps of Engineers (2011) Upper Mississippi River Navigation Charts, Chart No. 135
- 487 River Mile 136 142. Retrieved from
- 488 <u>http://www.mvr.usace.army.mil/Portals/48/docs/Nav/NavigationCharts/UMR/CHART\_135.pdf</u>
- 489 U.S. Geological Survey, 2017, National Water Information System data available on the World
- 490 Wide Web (USGS Water Data for the Nation), accessed August 21, 2017 at URL
- 491 http://waterdata.usgs.gov/nwis/.
- 492 van Heerden, I., and K. DeRouen, K., Jr. (1997) Implementing a barrier island and barrier
- 493 shoreline restoration program— the state of Louisiana's perspective: Journal of Coastal
- 494 Research, v. 13, p. 679–685.

- 495 van Schijndel S.A.H., and Kranenburg C. (1998) Reducing the siltation of a river harbour,
- 496 Journal of Hydraulic Research, 36:5, 803-814, DOI: 10.1080/00221689809498604
- 497 Wilkerson, G. V., and Parker, G. (2011) Physical Basis for Quasi-Universal Relationships
- 498 Describing Bankfull Hydraulic Geometry of Sand-Bed Rivers. Journal of Hydraulic Engineering,
- 499 137(7), pp. 738-753. DOI: 10.1061/(ASCE)HY.1943-7900.0000352.
- 500 Winterwerp, J.C. (2005) Reducing Harbor Siltation. I: Methodology. Journal of Waterway, Port,
- 501 Coastal, and Ocean Engineering 131(6) pp. 258-266. 10.1061/(ASCE)0733-
- 502 950X(2005)131:6(258)
- 503 Winterwerp, J.C., Eysink, W.D., Kruiningen, F.W., Christiansen, H., Kirby, R. and Smith, T.J.
- 504 (1994). The current deflecting wall: A device to minimize harbor siltation. Dock Harbor Authority.
- 505 74(849), pp. 243-247.
- 506 Wolanski, E., Asaeda, T., and Imberger, J. (1989) Mixing across a lutocline. Limnology and
- 507 Oceanography, 34(5), pp. 931-938.

508	Fig 1. Harbor Location. Figure prepared by the authors based on Navigation Chart No. 135 for
509	the Upper Mississippi River (US Army Corps of Engineers 2011).
510	
511	Fig. 2. Sedimentation patterns inside the harbor based on original harbor bathymetry and
512	bathymetric surveys at four different dates in 2009.
513	
514	Fig. 3. Sediment sampling locations. Samples 1-8 and the drum were extracted in December of
515	2010. Samples A, B and C were extracted in December of 2011.
516	
517	Fig. 4. Grain size distributions of harbor bed samples.
518	
519	Fig. 5. Picture taken during a settling column test. The left image shows the full column for a
520	test with a high initial concentration. Note how the concentration varied between the top and the
521	bottom of the settling column as shown in the right-most panes.
522 523	Fig. 6. Harbor bed sediment settling velocities.
524	
525	Fig. 7. Flow discharge and suspended sediment concentrations in the Mississippi River at St.
526	Louis, MO for the period Oct. 01, 1980 to Sep. 30, 2011.
527	
528	Fig. 8. Suspended sediment concentration rating curve for the Mississippi River at St. Louis,
529	MO. The dashed lines indicate envelopes for values equal to 0.2, 0.5, 2.0, and 5.0 times the
530	concentration values estimated with the power relation fit to the data shown in the lower right of
531	the figure.
532	
533	Fig. 9. Grain size distributions for the material in suspension in the Mississippi River at St.
534	Louis, MO. The solid gray lines correspond to the 108 available measurements; the solid black

535	line corresponds to the median grain size distribution and the dashed lines represent the 75 <sup>th</sup>
536	and $25^{th}$ percentiles. The bulk $D_{50}$ for the material is 0.008 mm.
537	
538	Fig. 10. Grain size distributions for the bed material in the Mississippi River at St. Louis, MO.
539	The solid gray lines correspond to the 114 available measurements; the solid black line
540	corresponds to the median grain size distribution and the dashed lines represent the 75 <sup>th</sup> and
541	$25^{th}$ percentiles. The bulk D <sub>50</sub> for the material is 0.44 mm.
542	
543	Fig. 11. Mean annual hydrograph and suspended sediment concentrations for Mississippi River
544	at St. Louis, MO. The dashed black line indicates the mean annual concentration values when
545	including the values observed in the period Feb. 22 <sup>nd</sup> - 26 <sup>th</sup> , 1985.
546	
547	Fig. 12. Mean annual flow (Q) duration curve and mean annual suspended sediment
548	concentration (C) duration curve for the Mississippi River at St. Louis, MO.
549	
550	Fig. 13. Effective discharge analysis plots and results. The left panel shows the flow frequency
551	curve; the middle panel shows the sediment rating curve; and the right panel shows the
552	weighted contributions and effective discharge.
553	
554	Fig. 14. Comparison of Harbor and Upper Mississippi River grain size distributions.
555	
556	Fig. 15. Flow and suspended sediment concentration conditions in the Upper Mississippi River
557	at St. Louis, MO for hydrologic years 2008-2010, as well as harbor siltation volumes and rates.
558	

# Table 1. Siltation volumes in the harbor and corresponding mean siltation rates for the period

Date	Excess Volume of Sediment	Volume Increase	Time	Mean Daily Siltation Rate <sup>1</sup>	Equivalent Deposit Thickness <sup>2</sup>	Avg. Daily Siltation Depth <sup>3</sup>
	[m³]	[m³]	[days]	[m³/day]	[m]	[cm/m²/day]
FEB 28 2008	166,444	-	-	-	-	-
AUG 13 2008	363,540	197,095	167	1,180	3.6	1.2
OCT 04 2008	337,617	0	52	0	3.4	0.0
JAN 26 2009	52,699	0	114	0	0.5	0.0
MAR 10 2009	57,271	4,572	43	106	0.6	0.1
JUN 04 2009	159,829	102,557	86	1,193	1.6	1.2
JUL 28 2009	211,052	51,224	54	949	2.1	1.0
DIC 16 2009	199,168	0	141	0	2.0	0.0
FEB 10 2010	207,347	8,178	56	146	2.1	0.2
JUN 02 2010	299,332	91,986	112	821	3.0	0.8

between February 2008 and June 2010.

<sup>1</sup> Mean siltation rate determined by dividing the volume increase between consecutive surveys by the number of days between them.

<sup>2</sup> Equivalent deposit thickness computed by dividing the excess volume of sediment by the total harbor area (500 m by 200 m - 10,000m<sup>2</sup>) assuming it is uniformly distributed.
 <sup>3</sup> Average daily siltation depth is computed by dividing the mean daily siltation rate by the total harbor

area.

561

562

	Date	Dec.	2010	Dec. 2011
	Sample(s)	Drum	1-9	A-C
	Volume	189 L	1.9 L	3.8 L
564				
565				

 Table 2. Harbor bed sediment sample dates and volumes.

Zone 1	Zone 2	Zone 3	Zone 4
Free settling	Flocculation settling	Hindered Settling	Consolidation
C < C <sub>1</sub>	$C_1 < C < C_2$	$C_2 < C < C_3$	C <sub>3</sub> < C
$w_s = w_{sf}$	$w_s = w_s(C)$	$w_s = w_s(C)$	$w_s \rightarrow 0$

# Table 3 Settling process zones

	Coefficient Value	a <sub>w</sub> 0.1	n <sub>w</sub> 2.1	b <sub>w</sub> 10	m <sub>w</sub> 2.08	C <sub>2</sub> [g/L] 10.1	w <sub>sm</sub> [m/s] 2.1e-4
570							
571							
572							

**Table 4.** Coefficients used in Eq. 1 to fit the measured harbor sediment settling velocities.

**Table 5.** Summary of available data from USGS gaging stations at St. Louis, MO and Chester,

IL that were used in the study.

Daily Data	Begin Date	End Da		
Discharge	01/01/1861	09/30/20		
Suspended sediment	10/01/1980	09/30/20		
Field/lab water-quality samples	01/31/1953	09/30/20		
USGS 07020500 Mississippi River at Chester, IL				
Daily Data	Begin Date	End Da		
Discharge	07/01/1942	09/30/20		
Suspended sediment concentration	10/01/1982	09/30/20		
Field/lab water-quality samples	10/14/1970	09/30/20		

# Table 6. Historic mean flows for Mississippi River at St. Louis, MO. Periods indicated

#### 578

# correspond to hydrologic years.

Period	[years]	1861-2011ª	1941-2011 <sup>b</sup>	1981-2011°	2007-2011 <sup>d</sup>
Mean Flow	[m³/s]	5,265	5,665	6,175	8,210

<sup>a</sup> Complete period of record for Mississippi River discharge at St. Louis, MO.

<sup>b</sup> Period of record matching the discharge data available at Chester, IL.

<sup>°</sup> Period of record matching the suspended sediment concentration measurements at St. Louis, MO

<sup>d</sup> Period of record beginning in the year when the harbor started operating (2007).

579

580

581	Table 7. Suspende	d sediment concentratio	on values measured in	1985 and associated flow
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discharges.

Date	Flow Discharge Q [m <sup>3</sup> /s]	Suspended Sediment Concentration C [g/L] <sup>a</sup>
Feb. 22 1985	6,343	2.75
Feb. 23 1985	11,836	5.74
Feb. 24 1985	15,348	6.72
Feb. 25 1985	17,302	5.69
Feb. 26 1985	18,632	3.09
• <b>T</b> L • • • th	· · · ·	

<sup>a</sup> The 99<sup>th</sup> percentile for concentrations measured in the period Oct. 1<sup>st</sup> 1980 to Sep. 30<sup>th</sup> 2011 is 1.78 g/L. Within that time period, only 3 (12, 30) values exceeded 4g/L (3 g/L, 2.5 g/L) corresponding to 0.03% (0.11%, 0.27%) of the data.

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# Table 8. Median $D_{50}$ values for Harbor and Upper Mississippi River sediment grain size

Sediment source		Median D <sub>50</sub> [mm]	
		Dispersed	Non-dispersed
Harbor	Drum	0.008	0.040
	S1-9	0.017	0.022
	A-C	0.015	0.050
River	Suspended	0.008	
	Bed	0.440	

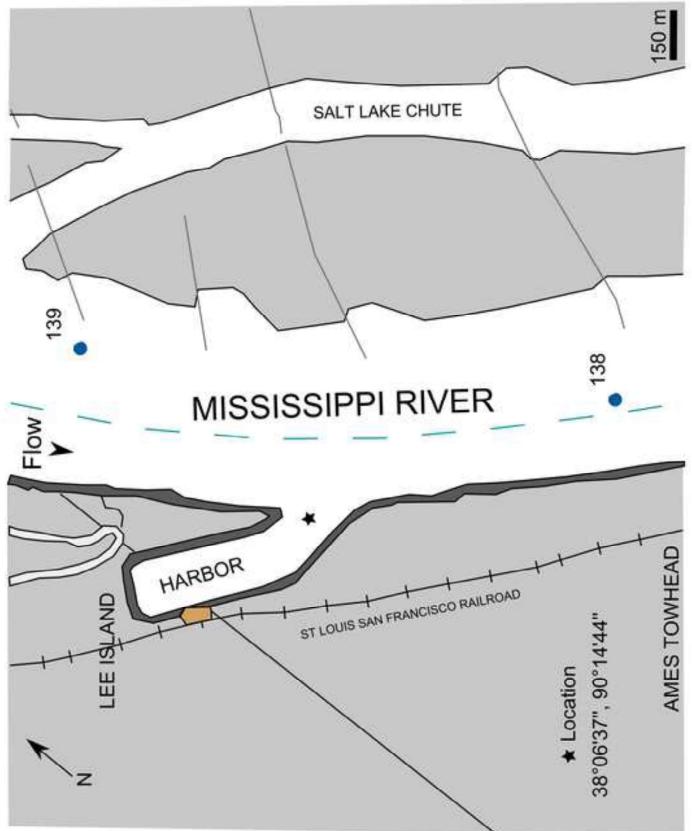
distributions.

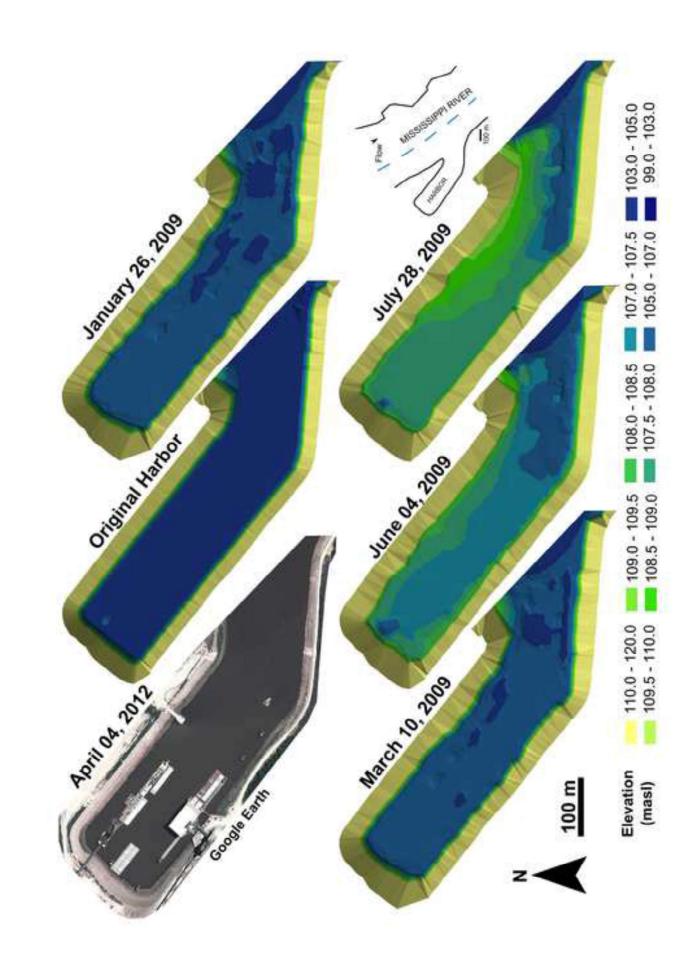
**Table 9.** Upper Mississippi River at St. Louis, MO characteristic discharges, exceedance and

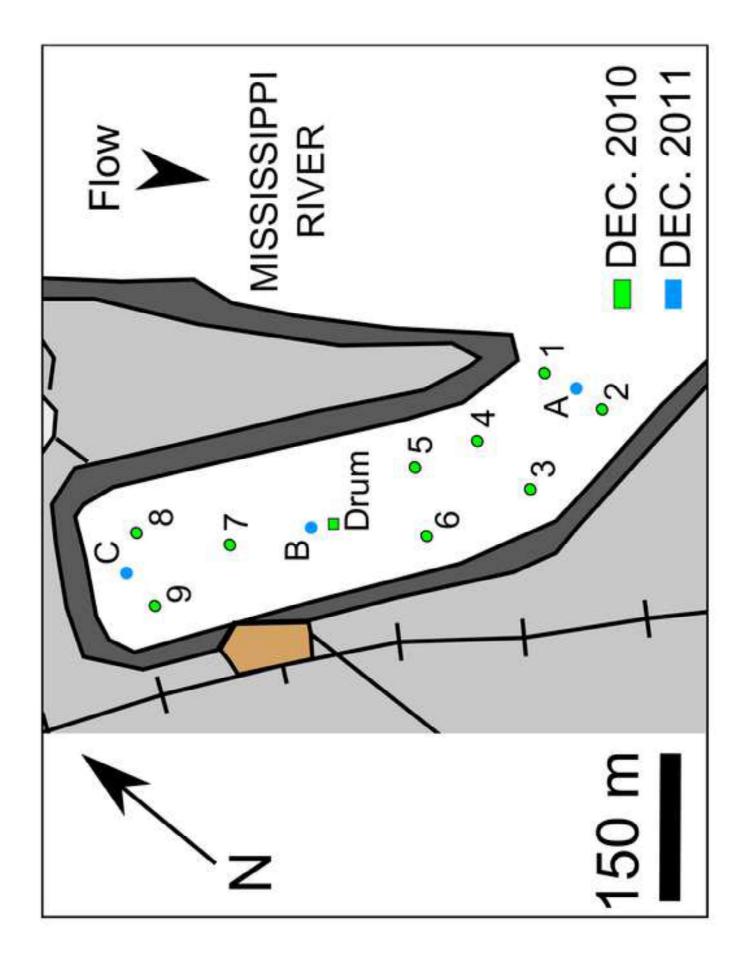
Discharge	Discharge Value	Exceedance	Associated Suspended Sediment
Туре	Q [m³/s]	[days - %]	Concentration C [g/L]
Mean	6,170	162 – 44%	0.264
Dominant	7,608	120 – 33%	0.337
Effective	9,582	36 – 10%	0.441

associated suspended sediment concentrations for hydrologic years 1981-2011.

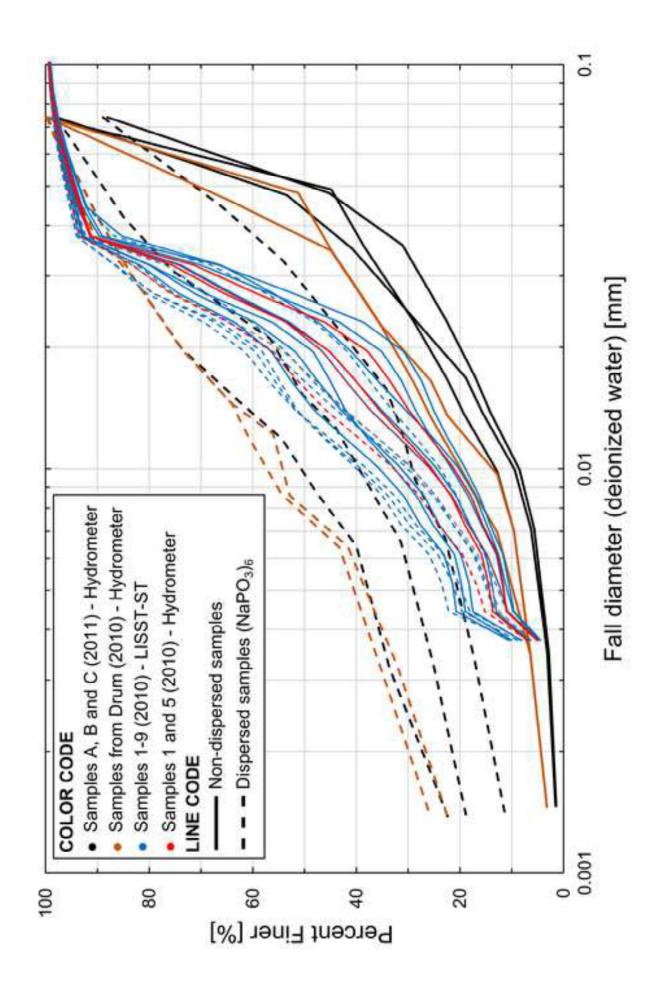


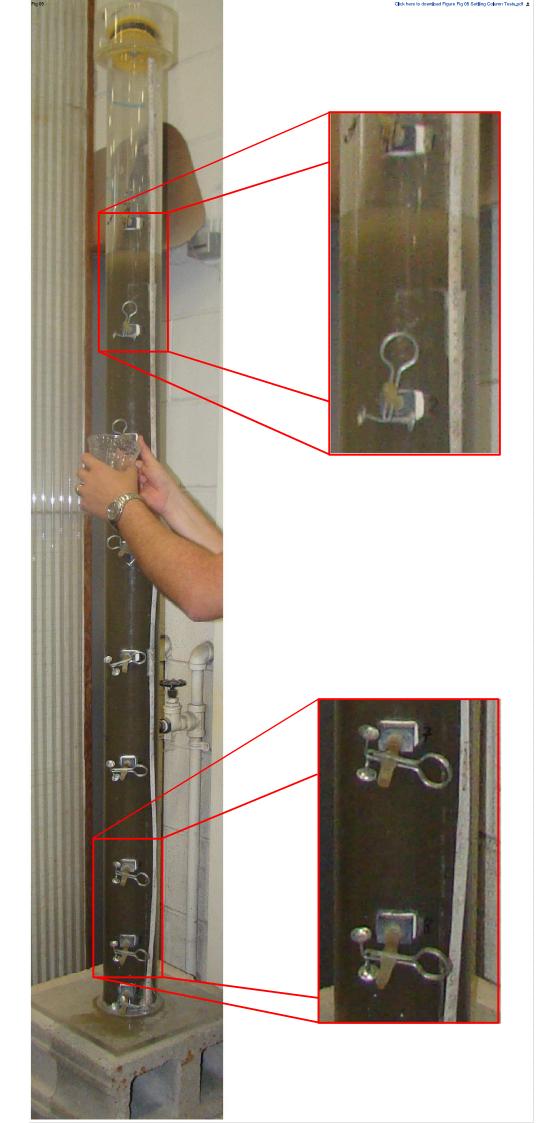




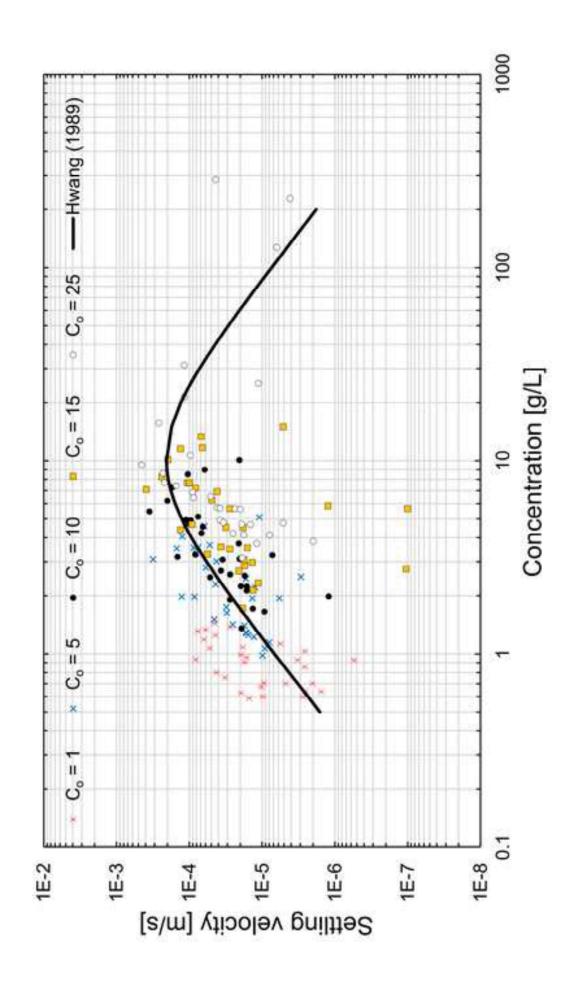


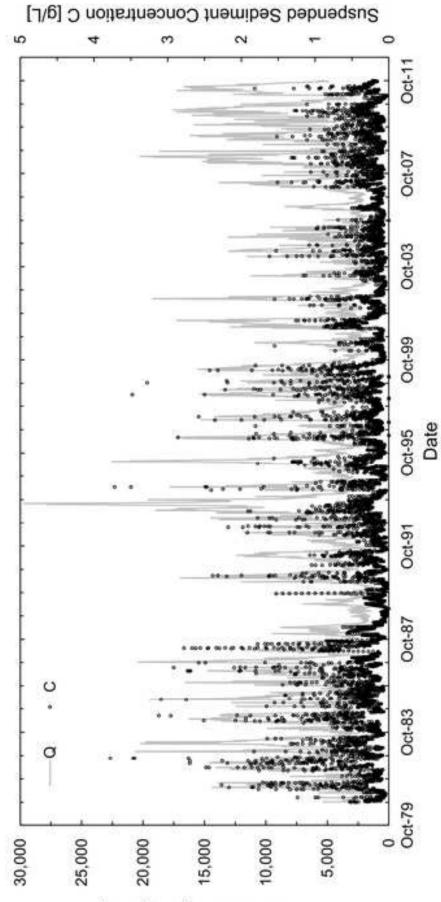












Flow Discharge Q [m3/s]

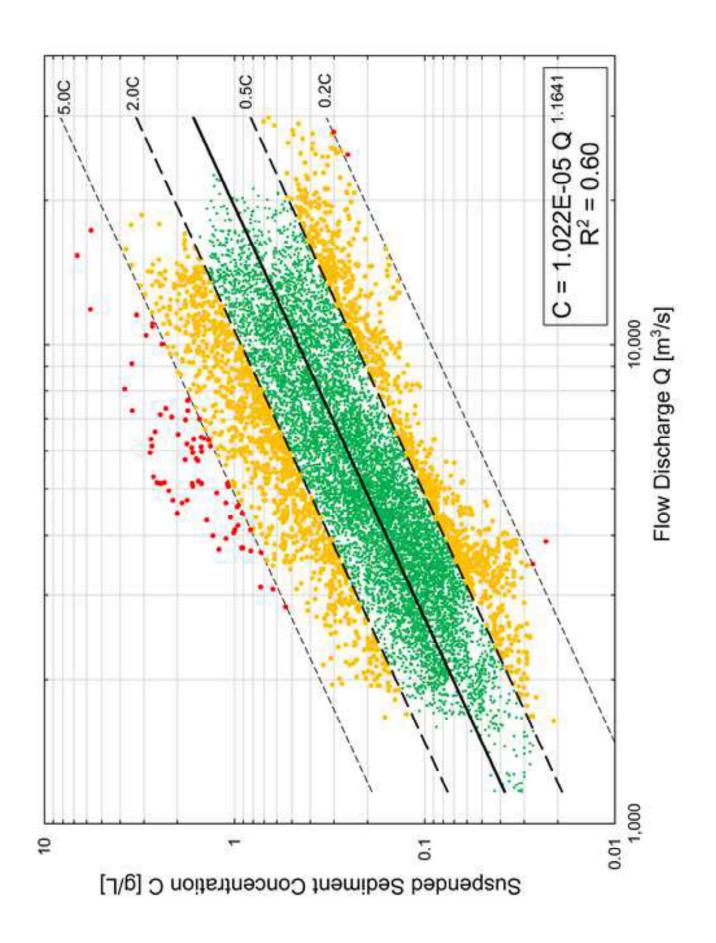
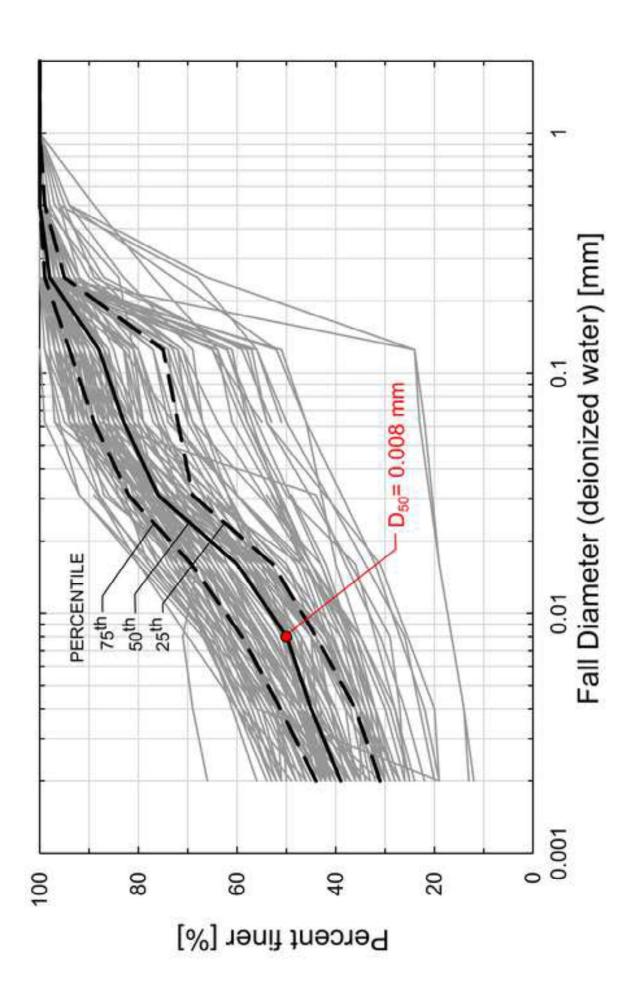
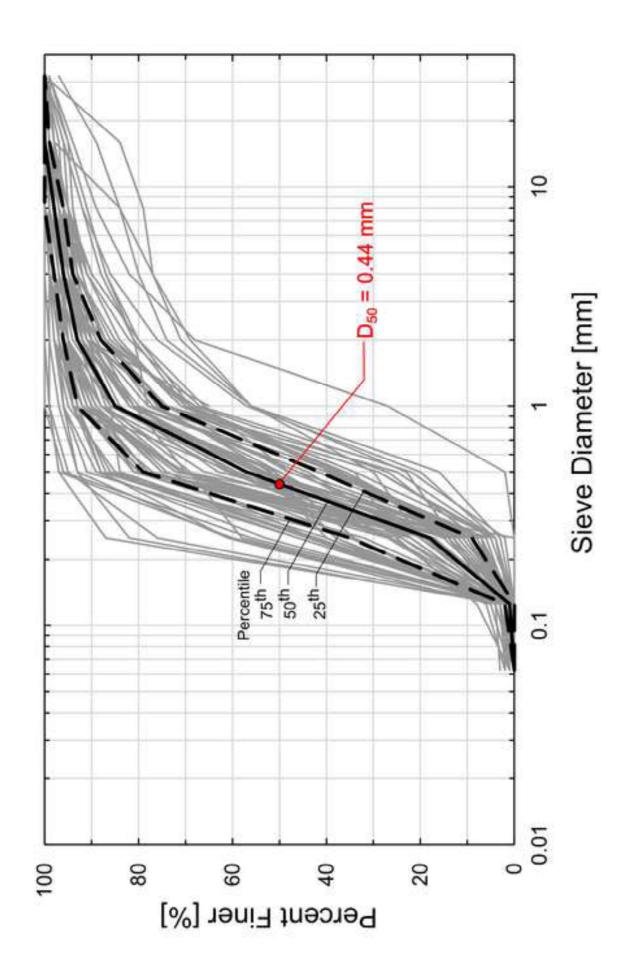


Fig 08







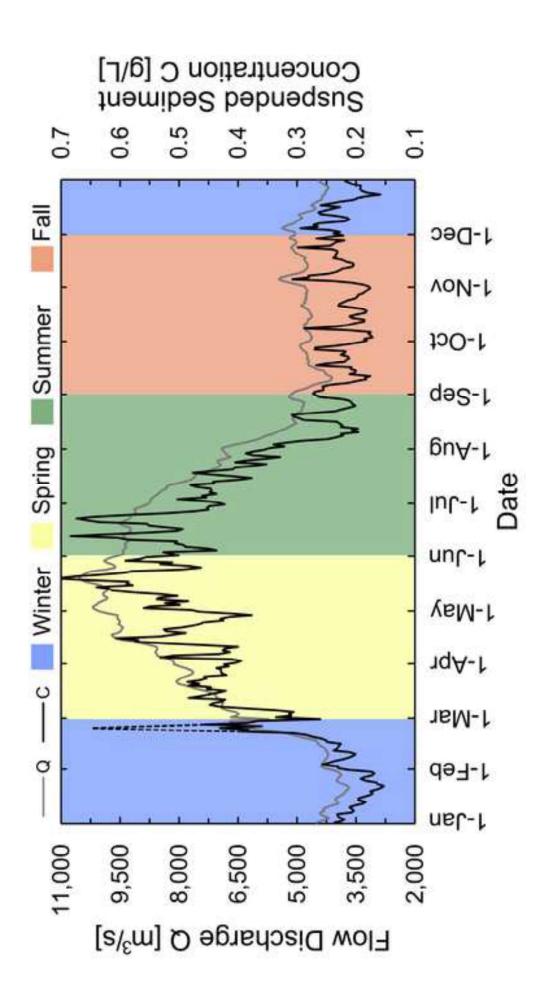
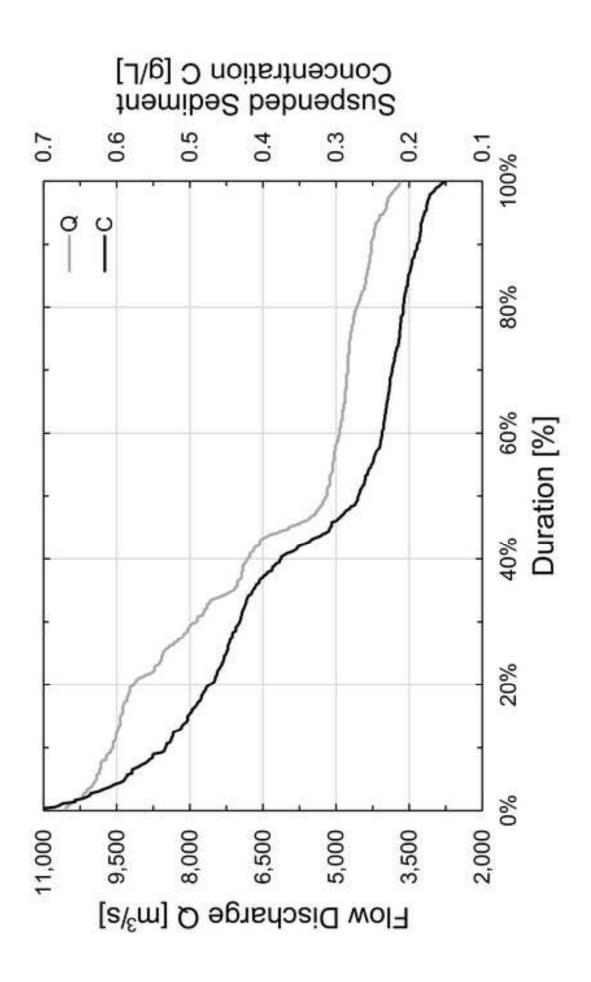
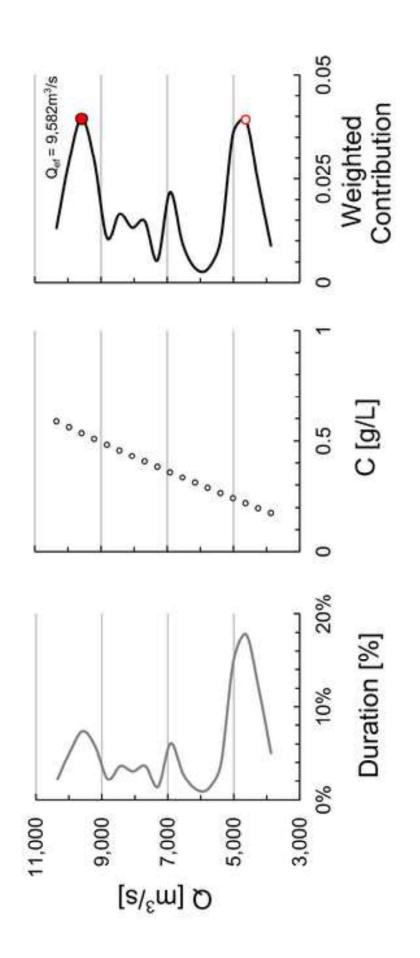


Fig 11

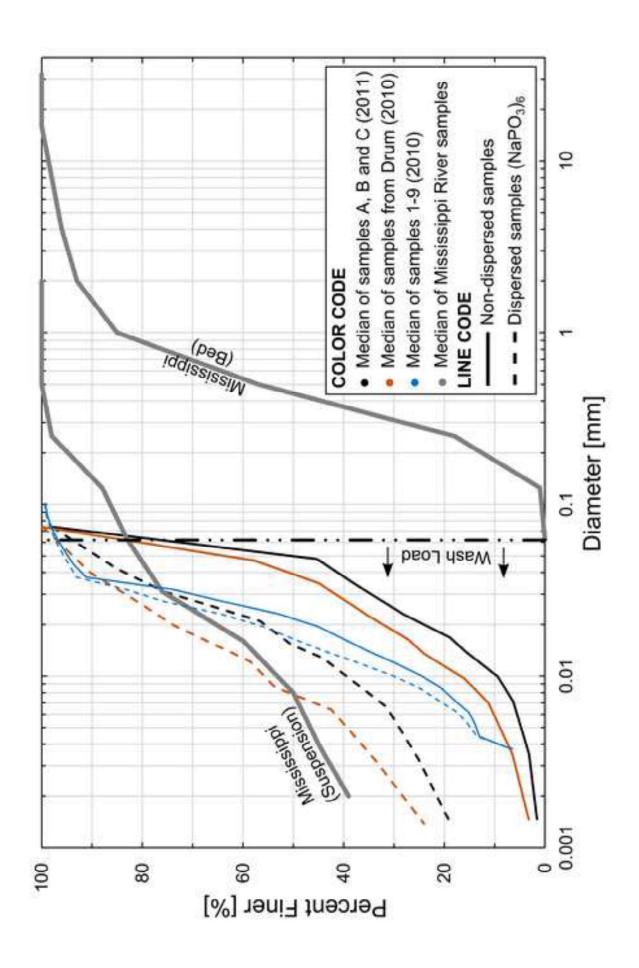












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